City and Borough of Juneau
Engineering Department
155 South Seward Street
Juneau, Alaska 99801
Telephone: 586-0490 FAX: 586-4530

ERRATUM No. 13

THE CITY AND BOROUGH OF JUNEAU PUBLICATION DECEMBER 2003 of the

STANDARD SPECIFICATIONS FOR CIVIL ENGINEERING PROJECTS AND SUBDIVISION IMPROVEMENTS

ERRATA DATE: **October 3, 2013**

The following items of the Specifications are modified as herein indicated. All other items remain the same. Please make the following corrections to your copy of the above mentioned Specifications.

***Add** the following Section:*

SECTION 01570 – EROSION AND SEDIMENT CONTROL

PART 1 - GENERAL

1.1 THE REQUIREMENT

A. The CONTRACTOR shall provide for erosion control during construction in accordance with the requirements of the Alaska Department of Environmental Conservation (ADEC). All discharge of pollutants and sedimentation from on-site drainage shall be caught on-site.

B. Erosion Control includes preparation and maintenance of a Storm Water Pollution Prevention Plan (SWPPP), control of erosion, sedimentation and discharge of pollutants, in accordance with the ADEC Construction General Permit (CGP).

C. The WORK under this section includes providing all labor, materials, tools and equipment necessary to construct and maintain temporary erosion control works; including but not limited to, wattles, silt fences, silt containment booms, settling ponds, check dams, ditches, etc.

PART 2 - PRODUCTS

2.1 MATERIALS

A. Materials shall be suitable for the intended use and perform effectively to control silt and surface erosion. All materials shall remain the property of the CONTRACTOR.

PART 3 - EXECUTION

3.1 GENERAL

A. The CONTRACTOR is responsible to prepare, submit and maintain a SWPPP, as required by the CGP, that is in accordance with their construction methodologies and sequences.

1) For projects disturbing greater than 1 Acre, this requirement shall include submission of a Notice of Intent (NOI) to ADEC prior to beginning of WORK. Copies of the NOI and SWPPP shall also be submitted to the ENGINEER within 5 days of submittal to ADEC.

2) For projects disturbing less than 1 acre, the SWPPP shall be submitted to the ENGINEER prior to the beginning of WORK; submittal to ADEC or an NOI are not required.

B. WORK at the Project site will not be permitted until the above documents are submitted to the ENGINEER and acceptance of this plan has been obtained from the governing agency or agencies (if required by the CGP).

C. The CONTRACTOR shall install temporary erosion control structures and devices as required by their SWPPP, prepared in accordance with the ADEC CGP. They shall be maintained in effective operating condition at all times. Prior to completion of work, the CONTRACTOR shall clean and remove all silt and debris from the settling pond and check dams.

D. Temporary erosion control structures shall remain in place until the project is completed and replaced by permanent erosion control WORK, protected by final stabilization or until the ENGINEER approves their removal.

E. The CONTRACTOR shall be responsible for meeting the requirements of all permits (including permits naming the OWNER, or other parties); therefore, shall be responsible for the quality of the run-off water from the Project site and for any fines and/or penalties resulting from the construction operation.

F. The CONTRACTOR shall submit NOT (Notice of Termination) at completion of the WORK and removal of all SWPPP items.

END OF SECTION

SECTION 01700 – PROJECT CLOSE-OUT, PART 1 - GENERAL, Article 1.3, FINAL SUBMITTALS, Paragraph A. ***Delete*** Items 6, 7 and 8 and ***replace with*** *the following sub-paragraph:*

1. Compliance Certificate and Release, signed by the CONTRACTOR, shall be submitted to the Engineering Contract Administrator.

SECTION 01700 – PROJECT CLOSE-OUT, PART 1 – GENERAL, Article 1.3, FINAL SUBMITTALS. ***Add*** *the following paragraph:*

C. Before final payment, the CONTRACTOR shall provide the Engineering Contract Administrator with clearance from the Alaska Department of Labor and Workforce Development for the CONTRACTOR and all Subcontractors that have worked on the Project. This clearance shall indicate that all Employment Security Taxes have been paid. A sample form for this purpose is at the end of Section 00800 – Supplementary General Conditions.

SECTION 01700 – PROJECT CLOSE-OUT, PART 1 – GENERAL. ***Replace*** *the* COMPLIANCE CERTIFICATE AND RELEASE FORM *with the form on the following page:*

COMPLIANCE CERTIFICATE AND RELEASE FORM

PROJECT: _____
CONTRACT NO: E_____

The CONTRACTOR must complete and submit this to the Engineering Contracts Administrator with respect to the entire contract.

Completed forms may be submitted upon completion of the Project. All requirements and submittals must be met before final payment will be made to the CONTRACTOR.

I certify that the following and any referenced attachments are true:

- All WORK has been performed, materials supplied, and requirements met in accordance with the applicable Drawings, Specifications, and Contract Documents.

- All Suppliers and Subcontractors have been paid in full with no claims for labor, materials or other services outstanding. If all Subcontractors and suppliers are not paid in full, please explain on a separate sheet.

- All employees have been paid not less than the current prevailing wage rates set by the State of Alaska (or U.S. Department of Labor, as applicable).

- All equal employment opportunity, certified payroll and other reports have been filed in accordance with the prime contract.

- The attached list of Subcontractors is complete (required from CONTRACTOR). The Contracts Administrator was advised and approved of all Subcontractors before WORK was performed and has approved any substitutions of Subcontractors.

- All DBE firms listed as a precondition of the prime contract award must have performed a commercially useful function in order for the WORK to count to a DBE goal. All DBE firms performed the WORK stated and have received at least the amount claimed for credit in the Contract Documents.

- All DBE Subcontractors must attach a signed statement of the payment amount received, the nature of WORK performed, whether any balance is outstanding, and indicate that no rebates are involved.

- If the amount paid is less than the amount originally claimed for DBE credit, the CONTRACTOR has attached approval from the Contracts Administrator for underutilization.

I understand it is unlawful to misrepresent information in order to receive a payment which would otherwise be withheld if these conditions were not met. I am an authorized agent of this firm and sign this freely and voluntarily. The foregoing statements are true and apply to the following project contractor.

_____ Capacity: CONTRACTOR
Firm Name

_____ _____ _____
Signed Printed Name and Title Date

Return completed form to: Engineering Contracts Administrator, City and Borough of Juneau, 155 South Seward Street, Juneau, AK 99801. Call (907) 586-0873 if we can be of further assistance or if you have any questions.

END OF SECTION

***Add** the following Section:*

SECTION 01704 – FINAL CLEAN-UP AND SITE RESTORATION

PART 1 - GENERAL

1.1 DESCRIPTION

A. The WORK under this Section includes providing all supervision, labor, materials, tools and equipment necessary for final clean-up and restoration of all areas disturbed by construction activities, to a condition equal to, or better than, before construction started. This does not include clean-up or restoration incidental to, or directly provided for by, other construction items.

PART 2 - PRODUCTS

2.1 MATERIALS

A. Any materials required shall conform to the appropriate section of these Specifications.

PART 3 - EXECUTION

3.1 CONSTRUCTION

A. The CONTRACTOR shall clean up all sites disturbed during construction of the Project. This includes removal of all construction equipment, disposal of all excess materials, disposal of all rubbish and debris, removal of all temporary structures, and grading of the sites so that no standing water is evident.

B. If the CONTRACTOR has obtained material from the CBJ/State pit, the excavated area shall be cleaned up and any stipulations required by the Individual Mining Plan shall be completed. The gravel pit overhead charge shall be paid to CBJ within 60 days after receiving the invoice from CBJ.

END OF SECTION

SECTION 02202 – EXCAVATION AND EMBANKMENT, PART 2 – PRODUCTS, ***delete*** *Article 2.7 SHOT ROCK BORROW and* ***replace with*** *the following:*

2.7 SHOT ROCK BORROW

A. Shot Rock Borrow shall consist of 10-inch minus shot rock and shall contain no mulch, frozen material, roots, sod or other deleterious matter. The shot rock borrow shall be evenly graded, with at least 10% by weight retained on the 8-inch screen. Shot rock will be obtained from quarry rock, unless otherwise approved by the ENGINEER.

B. Shot Rock Borrow shall have a plasticity index not greater than 6, as determined by AASHTO T90. It shall consist of not more than 6% by weight of particles that pass the No. 200 sieve as determined by ATM T-7. The percent of minus No. 200 material will be determined on minus 3-inch material.

C. At least 50% by weight of the particles retained on the 3/8-inch sieve shall have at least two fractured faces as determined by ATM T-4.

D. Elongation Specification:
The length of the crushed stone backfill shall not be more than twice the designated screen dimensions.

E. Sodium Sulfate Loss:
Aggregate shall pass the percent sodium sulfate loss per AASHTO T 104 with 9% maximum.

F. LA Abrasion:
Percent of wear per AASHTO T 96 shall be 45% maximum.

G. Shot Rock Borrow for this project shall have a maximum Nordic Abrasion value of 22. Test procedure for Nordic Abrasion is Alaska Test Method 312. This is available at the CBJ Engineering Department and State of Alaska Department of Transportation and Public Facilities Southeast Region Materials Laboratory.

2.8 2-INCH MINUS SHOT ROCK

A. 2-Inch Minus Shot Rock shall contain no mulch, frozen material, roots sod or other deleterious matter, and shall be obtained from a rock quarry, unless approved otherwise by the ENGINEER.

B. The shot rock shall have a plasticity index not greater than 6, as determined by AASHTO T 90. It shall consist of not more than 3% by weight of particles that pass the No. 200 sieve, as determined by ATM T-7.

C. At least 50% by weight of the particles retained on the 3/8-inch sieve shall have at least two fractured faces as determined by ATM T-4.

D. At least 70% by weight of particles shall be retained on the 1-inch sieve and 100% shall pass the 2-inch sieve.

E. Elongation Specifications:
The length of the crushed stone backfill shall not be more than twice the designated screen dimensions.

F. Sodium Sulfate Loss:
Aggregate shall pass the percent sodium sulfate loss per AASHTO T 104 with 9% maximum.

G. LA Abrasion:
Percent of wear per AASHTO T 96 shall be 45% maximum.

H. 2-Inch Minus Shot Rock for this Project shall have a maximum Nordic Abrasion value of 22. Test procedure for Nordic Abrasion is Alaska Test Method 312. This is available at the CBJ Engineering Department and State of Alaska Department of Transportation and Public Facilities Southeast Region Materials Laboratory.

SECTION 02202 – EXCAVATION AND EMBANKMENT, PART 3 - EXECUTION, Article 3.4, EMBANKMENTS CONSTRUCTED FROM ROCK FRAGMENTS, ***add*** *the following paragraphs:*

C. Shot Rock Borrow may be placed within the embankment in a single lift of 18-inches maximum.

D. All rock embankment surfaces shall be rolled full width with as many passes of a vibratory roller as required to obtain a solid mass of interlocking rock fragments, prior to placement of subsequent layers of material.

E. The surface of the shot rock borrow shall be sealed with fines from the shot rock materials, or shall have imported clean sand or other non-frost susceptible material used to seal the surface, as approved by the ENGINEER, before placement of the 2-inch minus shot rock base course. This work shall be considered incidental to other WORK under the contract.

SECTION 02202 – EXCAVATION AND EMBANKMENT, PART 3 - EXECUTION, ***add*** *the following Articles:*

3.6 SIDESLOPES AND DITCH GRADING

A. Sideslopes and Ditch Grading shall include all excavation, backfill, embankment construction, compaction, grading, and other work necessary to construct drainage swales, ditches, berms, and roadway sideslopes outside the edge of roadway, or outside other areas to be covered by asphalt pavement or concrete, as shown on the Drawings, in these Specifications, or as directed by the ENGINEER.

1. All areas beyond the roadway and 2-foot shoulders that are disturbed during construction shall be graded to a smooth, uniform grade and appearance, free of humps or low areas that cause standing water in excess of 1-inch in depth.

2. Sideslopes and Ditch Grading, which will include all grading of areas beyond the back of the 2-foot roadway shoulder and shall extend to the limits shown on the Drawings, or to the limits necessary to restore the driveways and disturbed areas to preconstruction conditions or better as shown on the Drawings or as directed by the ENGINEER.

3.7 2-INCH MINUS SHOT ROCK w/BASE COURSE

A. The full depth of 2-inch minus shot rock shall be graded to a uniform surface and compacted with a vibratory roller prior to placing base course, Grading D-1. No base course, Grading D-1 shall be placed until the 2-inch minus shot rock layer has been approved by the ENGINEER.

3.8 INDIVIDUAL MINING PLAN

A. If the CONTRACTOR decides to use material form the CBJ/State Lemon Creek Borrow Pit or Stablers Point Rock Quarry, the CONTRACTOR shall provide an Individual Mining Plan that conforms to the requirements of Section 00700 - General Conditions, Article 4.6.

B. The Individual Mining Plan shall be developed using the survey information provided by the OWNER, or the CONTRACTOR may provide an independent survey with 2-foot contours of the Pit and Quarry property. The survey shall provide sufficient survey information to calculate quantities, show drainage features and property boundaries. If the CONTRACTROR uses the OWNER furnished survey information, the Individual Mining Plan shall be done in AutoCAD format.

SECTION 02401 – SANITARY SEWER PIPE, PART 2 - PRODUCTS, ***add*** *the following Article:*

2.9 PIPE CONNECTORS

A. "Mission Flex Seal" connectors will not be acceptable for use on this project.

SECTION 02402 – SANITARY SEWER MANHOLES AND CLEANOUTS, PART 2 - PRODUCTS, Article 2.3, MISCELLANEOUS, ***delete*** *Paragraph F and* ***add*** *the following:*

F. Manhole exterior joint waterproofing shall be a Miradri system as manufactured by Carlisle CCW, including Carlisle – CCW 704 primer, CCW Miradri 861 Membrane, and CCW 704 mastic, or approved equal that includes a membrane and adhesive system for positive water exclusion. The membrane shall extend at least 18-inches each side of manhole joints, except this width may be reduced to 9-inches each side of manhole joints if the joint is less than 4-feet below finish grade and the joint is above the maximum water table.

G. Delete the requirement for the flexible annular space filler, as shown on CBJ Standard Detail 209 – Manhole Connection Details, for the Flexible Seal Adapter.

H. Manhole Grade Ring Adjustment Units:

1. Manhole grade adjustment units shall be Recycled Adjustment Risers, "Infra-RISER," as manufactured by GNR Technologies, or approved equal.

2. The adjustment riser shall consist of no less than 80% by weight recycled rubber from tires, and no less than 10% by volume shredder fiber. The riser shall meet or exceed the following when tested on units not less than 24 hours old, and not more than 60 days old, and maintained at 23±2°C (73±3°F) for at least 12 hours prior to and during testing.

Physical Property	**Test Method**	**Acceptable Results**
Density	ASTM C642-90	1.098±0.05g/cm^3
Durometer hardness - molded surface	GNR method based on ASTM D 2240	75A±5 points
Durometer Hardness - interior surface	GNR method based on ASTM D 2240	73A± 5 points
Tensile Strength	ASTM 412-87	1.6 MPa (232 pai) (not < 1 Mpa)
Compression Deformation - initial deformation	GNR method based on ASTM D 575	under 1 MPa (145 psi) 6±2%
Compression Deformation - final deformation	GNR method based or ASTM D 575	under 1 MPa (145 psi) 6±2%
Compression Set	GNR method based On ASTM 395	under 1 MPa (145 psi) 0.4% (=4% max.)
Brittleness at low temperature	ASTM D 746-79	-40° F (-40°C)
Freeze/Thaw when exposed to deicing chemicals	ASTM 672-91	no loss after 50 cycles
Coefficient of thermal Expansion	ASTM C 531-85	1.6 X 10^4 mm/mm/°C (8 X 10^5 in/in/°F)
Weathering 70 hr. @ 70°C	ASTM D 573-88	
- hardness retained		100%
- compressive strength retained		100%
- tensile strength retained		100%
- elongation retained		100%

3. Each adjustment riser shall be clearly marked on the inside surface with the manufacturer's name and location of the manufacturer.
4. The manufacturing process shall be such that individual units will be consistent in quality and appearance. All rough edges shall be trimmed prior to shipping.
5. The thickness of the adjustment riser shall be within 3 mm of the manufacturer's stated dimensions. All other dimensions shall be within 5 mm.

6. Except for shim or wedge units, the deviation from the plane parallel to the theoretical surface shall not be greater than 1 in 500.

SECTION 02402 – SANITARY SEWER MANHOLES AND CLEANOUTS, PART 3 - EXECUTION, Article 3.1 CONSTRUCTION, ***delete*** *paragraphs M through R and* ***replace*** *with the following paragraphs M – O:*

M. Manhole Grade Ring Adjustment Units are required for each new sanitary sewer manhole, reconstructed sanitary sewer manhole, and adjustment of existing manhole to grade.

1. Each manhole shall contain at least one recycled rubber riser, with thickness varying to match frame and cover to finish grade requirements, to form the final surface for installation of the frame.
2. The total height of the rubber adjustment riser shall be a minimum of 1" and a maximum of 3".
3. Concrete and steel surfaces to receive sealing compound shall be clean, dry and free of grease or oils.
4. Adjustment risers shall be bonded to adjacent surfaces by laying a continuous bead, 5/16" thick cold applied joint sealant compound conforming to ASTM-D 1850 (PL Premium POLYURETHANE Door, Window & Siding Sealant *or* PL Premium POLYURETHANE Concrete & Masonry Sealant, formerly Chemrex CX-22) or equivalent, on the top surface of the concrete course, or the bottom surface of the riser, on a diameter 1" smaller than the outside diameter of the rubber adjustment riser.
5. The adjustment riser shall then be seated firmly in place, ensuring it is centered over the opening. Apply a second continuous strip of sealant to the top surface of adjustment riser, 0.5" from the outside diameter of the rubber adjustment riser or manhole frame.
6. The adjustment riser must form the final surface for the seating of the frame and cover assembly. Concrete adjustment units must not form the final surface for seating the frame.
7. If more than one adjustment riser is required, a continuous bead of sealant shall be applied between each unit in the same manner as in paragraph 4 above. A continuous bead of sealant shall also be placed on the top surface of the concrete course or on the bottom surface of the bottom riser and to the top surface of the top adjustment riser.
8. The frame shall then be set firmly in place ensuring that it is properly centered over the structure opening and is firmly contacting the rubber riser through the sealant.
9. Adjustment risers shall have an inside diameter that is within 2" of the inside diameter of the concrete structure, and equal to the outside diameter of the concrete structure ±2".

N. Manhole frames and covers shall be set to final grade prior to final paving operations, with the compacted pavement to provide a depression to the top of manhole frame within the allowable limits of 3/8-inch minimum to 3/4-inch maximum, as determined by using an 8-foot long straight edge across the frame in all directions.

1. The frame can be set to final position prior to the laydown machine passing over the structure, or immediately following the laydown machine passing over the structure.
2. The intended purpose of these requirements is that the asphalt pavement is compacted to grade around the frame and cover with no cut out of compacted pavement allowed.
3. If the depression of the frame and cover below finish pavement is found to be out of allowable tolerances after the pavement has cooled to the point that sawcutting and removal of the pavement is necessary, the following corrective action will be required:

    a. A square cut-out of the pavement shall be made to a minimum of 6-inches and maximum of 8-inches outside the edge of frame flange, with this cut-out oriented with the sides at 45° to traffic.
    b. A concrete transition slab shall be constructed as shown in the detail on the Drawings. This slab shall be allowed to cure for a minimum of 48 hours before placing the hot asphalt mix over the transition slab.
    c. This WORK shall be completed prior to the street fog sealing operation.

O. Manhole riser rings shall be sealed to the top of manhole cone or flattop and to each other with one run of "RAM-NEK" or "RUB-R-NEK" around the inside edge and one run around the outside edge of the riser ring. The units shall be heated and compressed to at least 50% of original thickness of the "RAM-NEK" or "RUB-R-NEK." No grout shall be used to seal the riser rings.

SECTION 02501 – STORM SEWER PIPE, PART 2 - PRODUCTS, ***add*** *the following Article:*

2.9 UNDERGROUND MARKING TAPE

A. Underground Marking Tape shall be yellow, at least 4-inches wide, 4-mil thick, polyethylene tape with a metallic backing capable of being traced with locators. The tape shall have black letters with the following wording: "Caution: Storm Sewer Line Buried Below," or similar. The marking tape shall be installed 12-inches above the top of all storm sewer mains and services.

SECTION 02502 – STORM SEWER MANHOLES, INLETS AND CATCH BASINS, PART 3 - EXECUTION, Article 3.1, CONSTRUCTION, ***delete*** *paragraph C and* ***replace*** *with the following paragraph C:*

C. Metal frames shall be set over the cast-in-place concrete support structure with a maximum ¼-inch thick mortar bed.

SECTION 02602 – VALVES, PART 2 – MATERIALS, Article 2.3, VALVE BOXES, *Paragraph A,* ***delete*** *the last sentence and* ***replace*** *with the following:*

The valve box base section shall be an East Jordan Iron Works 8555 30-B or 36-B slide valve box bottom section, or approved equal. The valve box top section shall be an East Jordan Iron Works 8555 Slide 26T, 16T or 10T valve box top or approved equal.

SECTION 02603 – FIRE HYDRANTS, PART 2 – PRODUCTS, Article 2.1, FIRE HYDRANTS, paragraph F. ***Delete*** *the first sentence and* ***replace*** *with the following*:

Fire hydrants shall be three-way and furnished with two 2 ½ inch hose nozzles and one 5-inch pumper nozzle. The pumper nozzle shall be one-piece design, compatible with 5-inch Storz hose coupling. The nozzle shall be an integral part of the fire hydrant assembly, resistant to tamper or removal by persons not familiar with the art of fire hydrant construction. Add-on Storz compatible adapters shall not be acceptable.

SECTION 02605 – WATER SERVICES, PART 2 – PRODUCTS, Article 2.1, WATER SERVICES, ***delete*** *paragraph C and* ***replace*** *with the following paragraph C*:

C. Water service pipe and materials shall be cold drawn, seamless annealed Type K Copper. Fittings for water pipe less than 2-inches in diameter shall be flared bronze fittings. Fittings for 2-inch pipe shall be bronze grip-lock compression fittings.

SECTION 02605 – WATER SERVICES, PART 3 – EXECUTION, Article 3.1, CONSTRUCTION, ***Add*** *the following paragraphs*:

E. Thaw wires shall be placed over a 6-inch minimum layer of backfill so the thaw wire does not come in contact with copper tubing. When two or more services are placed in the same trench, thaw wires shall have a minimum 6-inch clearance between adjacent thaw wires.

F. Thaw wires shall be run into the service box near the top of box through a drilled hole large enough for the thaw wire. No cutting or notching of the service box will be permitted.

Add *the following Section:*

SECTION 02607 – PIPE INSULATION

PART 1 - GENERAL

1.1 DESCRIPTION

A. The WORK under this Section includes providing all labor, materials, tools and equipment necessary for furnishing and installing pipe insulation for water pipe and service pipe at locations shown on the Drawings and as directed by the ENGINEER.

PART 2 - PRODUCTS

2.1 RIGID INSULATION

A. Rigid insulation shall be rigid board closed cell polystyrofoam material containing a flame retardant additive specifically designed for underground pipe or pavement installations, equivalent to Dow Chemical Company Styrofoam HI, and approved by the ENGINEER.

2.2 SPRAYED-ON INSULATION

A. Sprayed-on urethane foam insulation applied directly to the pipe exterior with an elastomeric coating, may be approved by the ENGINEER, provided the material has demonstrated a satisfactory performance history in underground installation and has the following physical properties:

Density	2 pcf, Minimum
Compressive Strength (ASTM D 1621)	35 psi, Minimum at 5% Deflective or Yield
Water Absorption (ASTM C 177)	0.25% by Vol. Maximum
Thermal Conductivity (ASTM C 177)	$\frac{\text{Max. 0.23 BTU}}{\text{Hr. Ft.}^2 \text{ EF. In. Thickness}}$

PART 3 - EXECUTION

3.1 CONSTRUCTION

A. When water pipes or service pipes have less than 5-feet of cover to finished grade or vertical clearance at a culvert crossing, either above or below, they shall be insulated as shown on CBJ Engineering Standard Detail 412 – Rigid Insulation, or as directed by the ENGINEER.

B. Rigid insulation shall be a minimum of 2-feet wide and 2-inches thick. The length of insulation required shall be as shown on the Drawings or as directed by the ENGINEER. Insulation shall be placed between 1 and 12-inches from the water pipe or service pipe with the width centered on the longitudinal axis of the water pipe or service pipe as shown on CBJ Engineering Standard Detail 412 – Rigid Insulation, or as directed by the ENGINEER.

C. Sprayed-on urethane foam insulation shall be a minimum of 4-inches thick and be installed in strict conformance to the manufacturer's recommendations. Precautions to protect CONTRACTOR personnel, Project inspectors, and the public in general shall be taken by the CONTRACTOR in compliance with OSHA Standards and the manufacturer's recommendations.

END OF SECTION

SECTION 02801 – ASPHALT CONCRETE PAVEMENT, PART 3 - EXECUTION, Article 3.8, SPREADING AND PLACING, ***delete*** *paragraph H and replace with the following:*

H. Manholes, cleanouts and water valve boxes shall be raised to grade prior to paving the final lift. The structures shall have no less than 3/8” and no greater than 3/4” depression from adjacent asphalt to top of the lid. Structures not meeting tolerances will be repaired as per CBJ Standard Detail 126 – CONCRETE COLLAR.

SECTION 02801 – ASPHALT CONCRETE PAVEMENT, PART 3 - EXECUTION, Article 3.10, JOINTS, ***add*** *the following paragraph:*

J. All joints with existing asphalt pavement shall be resealed with asphalt cement after the new pavement has cooled to ambient temperature. All joints with concrete gutters found to have a gap shall be blown out using a weed burner torch, filled with asphalt cement and covered with a layer of dry sand. Excess sand shall be removed and asphalt cement placed on the concrete gutter more than one-inch from the edge of gutter shall be removed using solvent or other approved methods.

SECTION 02803 - FOG SEAL COAT, PART 2 - PRODUCTS, Article 2.1, MATERIALS, ***revise*** *paragraph C to read as follows:*

C. The blotter material shall be suitable, dry, clean sand.

By: _____

John Bohan, P.E.
Chief CIP Engineer

Total number of pages contained in Errata Sheet: 14